

**REVISED EXPRESS TERMS–15-DAY MODIFICATIONS  
FOR  
PROPOSED BUILDING STANDARDS  
OF THE  
STATE HISTORICAL BUILDING SAFETY BOARD  
REGARDING PROPOSED CHANGES TO  
CALIFORNIA HISTORICAL BUILDING CODE  
CALIFORNIA CODE OF REGULATIONS, TITLE 24, PART 8  
(SHBSB 01/10)**

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The State Historical Building Safety Board (SHBSB) proposes to make necessary changes to the 2010 edition of the California Historical Building Code.

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**LEGEND FOR EXPRESS TERMS**

1. **Existing California text or language being modified:** All such language shown in normal Arial 10 point; modified language is underlined or shown in ~~strikeout~~.
  2. **Repealed text:** All such language shown in ~~strikeout~~.
  3. **Amended, adopted or repealed language after public comment period:** All such language shown in double underline or ~~double strikeout~~.
  4. **Notation:** Authority and Reference citations are provided at the end of each section.
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**EXPRESS TERMS**

**Item 7**

**8-705.2.1** Any un-safe conditions in the lateral-load-resisting system shall be corrected, or alternative resistance shall be provided. When strengthening is required ~~Additional~~ resistance shall be provided to meet the minimum requirements of ~~this code, the CHBC.~~ The strengthening measures shall be selected shall fulfill with the intent to of prevent partial or total structural collapse and protect occupants from life threatening injury meeting the performance objectives set forth in Section 8-701.2. The evaluation of structural members and structural systems for seismic loads shall consider the inelastic performance of structural members and their ability to maintain load-carrying capacity during ~~extreme the seismic loadings greater than~~ prescribed by the ~~CHBC regular code.~~

**Rationale for change:**

SHBSB proposes to amend Section 8-705.2.1 based on a stakeholder comment recommending that the new language not supersede or exceed the goals laid out in 8-702.1. The new language in Section 8-705.2.1 could be read so as to require somewhat more stringent conformance to those goals. The amended text reflects the recommendation of the commenter.

Notation:

Authority: Health and Safety Code Section 18959.5

Reference: Health and Safety Code Sections 18959 and 18959.5

**Item 8**

**8-706.1 Lateral Loads Seismic Forces.** ~~The Strength-level seismic forces used to evaluate the structure for resistance to wind and seismic loads need not exceed 0.75 times the seismic forces prescribed by the 1995 Edition of the California Building Code (CBC) regular code requirements. Furthermore, the seismic base shear need not exceed 0.40W for structures with Occupancy Category III & IV and 0.30W for structures with Occupancy Category I or II. Near fault increases in ground motion (maximum considered earthquake ground~~

~~motion of 0.2 second spectral response greater than 150% at 5% damping) shall be considered for historic structures within Occupancy Categories I, II or III only when the fundamental period of the building exceeds 0.5 seconds. The seismic forces may shall be computed based on the  $R_w$   $R$  values tabulated in the regular code for similar lateral-force-resisting-systems including consideration of the structural detailing of the members where such R values exist. Where such R values do not exist, an appropriate R value shall be rationally assigned considering the structural detailing of the members. All deviations of the detailing provisions of the lateral force resisting systems shall be evaluated for stability and the ability to maintain load-carrying capacity at increased lateral loads.~~

Exceptions:

1. The forces need not exceed 0.75 times the seismic forces prescribed by the regular code requirements.
2. For Occupancy Category I, II or III structures, near-fault increases in ground motion (maximum considered earthquake ground motion of 0.2 second spectral response greater than 150% at 5% damping) need not be considered when the fundamental period of the building is 0.5 seconds in the direction under consideration.
3. For Occupancy Category I or II structures, the seismic base shear need not exceed 0.30W.
4. For Occupancy Category III of IV structures, seismic base shear need not exceed 0.40W.

**Rationale for change:**

SHBSB proposes to amend Section 8-706.1 based on a stakeholder comment recommending that the section be reformatted. The four numbered items proposed by the commenter are intended to eliminate possible ambiguity in the interpretation of the section.

**Notation:**

Authority: Health and Safety Code Section 18959.5  
Reference: Health and Safety Code Sections 18959 and 18959.5

**Item 9**

**8-706.1.1** When a building is to be strengthened with the addition of a new lateral force resisting system, the R value of the new system can be used when the new lateral force resisting system resists at least 75% of the building's base shear regardless of its relative rigidity.

**Rationale for change:**

SHBSB proposes to amend Section 8-706.1.1 based on a stakeholder comment recommending

**Item 10**

**8-706.1.2** Un-reinforced masonry bearing wall buildings shall comply with the California Existing Building Code (CEBC), Appendix Chapter A1, 2010 Edition, of the Uniform Code for Building Conservation (UCBC), 1994 edition Appendix A, Chapter A1 of the International Existing Building Code, (IEBC), 2006 edition, and as modified by this code the CHBC. Reasonably equivalent Alternative standards may be used on a case-by-case basis when approved by the authority having jurisdiction. It shall be permitted to exceed the strength limitation of 100 psi in Section A108.2 of the CEBC when test data and building configuration supports higher values subject to the approval of the authority having jurisdiction

**Rationale for change:**

SHBSB proposes to amend Section 8-706.1.2 based on a stakeholder comment recommending adding the correct reference to the current addition of the IEBC and allowing consideration of increased strength limitation based on testing.

**Notation:**

Authority: Health and Safety Code Section 18959.5  
Reference: Health and Safety Code Sections 18959 and 18959.5

**Item 11**

**8-706.1.3** All deviations ~~of from~~ the detailing provisions of the lateral-force-resisting systems shall be evaluated for stability and the ability to maintain load-carrying capacity at ~~increased lateral loads~~ the expected inelastic deformations.

**Rationale for change:**

SHBSB proposes to amend Section 8-706.1.3 based on a stakeholder comment recommending editorial change and ligning deviations from detailing provisions with inelastic deformations.

**Notation:**

Authority: Health and Safety Code Section 18959.5

Reference: Health and Safety Code Sections 18959 and 18959.5

**Item 12**

**8-706.2.1** All structural materials or members that do not comply with detailing and proportioning requirements of the regular code shall be evaluated for potential seismic performance and the consequence of non-compliance. All members ~~which might~~ that would be reasonably expected to fail and lead to ~~possible collapse,~~ or ~~threaten life safety~~ life threatening injury when subjected to seismic demands ~~in excess of those prescribed in Section 8-706.1,~~ shall be judged unacceptable and appropriate structural strengthening shall be developed. ~~Anchorage for veneers and decorative ornamentation shall be included in this evaluation.~~

**Rationale for change:**

SHBSB proposes to amend Section 8-706.2.1 in response to a stakeholder comment stating the changes in this section could similarly be interpreted to include consideration of seismic demands in excess of those required by the regular code for new buildings. The changes address this concern.

**Notation:**

Authority: Health and Safety Code Section 18959.5

Reference: Health and Safety Code Sections 18959 and 18959.5

**Item 15**

**SECTION 8-802  
GENERAL ENGINEERING APPROACHES**

~~Allowable stresses or ultimate strength values for archaic materials shall be assigned based upon similar conventional codified materials, or on tests as hereinafter indicated. The archaic materials and methods of construction shall be thoroughly investigated for their details of construction in accordance with Section 8-703. Testing shall be performed when applicable to evaluate existing conditions. The architect or structural engineer in responsible charge of the project shall assign allowable stresses or ultimate strength values levels to archaic materials. Such assigned allowable stresses, or ultimate strength values, shall not be greater than those provided for in the following sections without adequate testing, and shall be subject to the concurrence of the enforcing agency.~~

**Rationale for change:**

SHBSB proposes to amend Section 8-802 based on a stakeholder comment recommending the change from Allowable Stresses, a previous model code term no longer used in the 2009 IBC.

**Notation:**

Authority: Health and Safety Code Section 18959.5

Reference: Health and Safety Code Sections 18959 and 18959.5

**Item 16**

**SECTION 8-804  
ALLOWABLE CONDITIONS FOR SPECIFIC MATERIALS**

Archaic materials which exist and are to remain in qualified historical buildings or structures shall be evaluated for their condition and for loads required by this code. The structural survey required in Section 8-703 of ~~this code~~ the CHBC shall document existing conditions, reinforcement, anchorage, deterioration and other factors pertinent to establishing allowable stresses, strength levels and ultimate stresses and adequacy of the archaic materials. The remaining portion of this chapter provides additional specific requirements for commonly encountered archaic materials.

**Rationale for change:**

SHBSB proposes to amend Section 8-804 based on a stakeholder comment recommending elimination of the term “ultimate strength” in the code.

**Notation:**

Authority: Health and Safety Code Section 18959.5

Reference: Health and Safety Code Sections 18959 and 18959.5

**Item 17**

**8-805.1 Existing Solid Masonry.** Existing solid masonry walls of any type, except adobe, may be allowed, without testing, a maximum ~~value of three~~ ultimate strength of nine pounds per square inch (~~20.7~~ 62.1 kPa) in shear where there is a qualifying statement by the architect or engineer that an inspection has been made, that mortar joints are filled and that both brick and mortar are reasonably good. The ~~allowable~~ shear stress above applies to un-reinforced masonry, except adobe, where the maximum ratio of un-supported height or length to thickness does not exceed ~~42~~ 13, and where minimum quality mortar is used or exists. Wall height or length is measured to supporting or resisting elements that are at least twice as stiff as the tributary wall. Stiffness is based on the gross section. ~~Allowable~~ Shear stress may be increased by the addition of 10 percent of the axial direct stress due to the weight of the wall directly above. Higher quality mortar may provide a greater shear value and shall be tested in accordance with ~~UBC Standard 21-6 as referenced in the 1997 UBC Appendix A, Chapter A1 of the International California Existing Building Code (ICEBC) 2006~~ 10 edition, and as modified by the CHBC.

**Rationale for change:**

SHBSB proposes to amend Section 8-805.1 based on a stakeholder comment recommending it be consistent with the change of reference from the 1997 UBC to CEBC Appendix A, Chapter A1. The nine pounds per square inch is a low number that can be used without testing. From allowable stresses to strength is a change by a factor of three. Going from three to mine is consistent with this change. This is a value that can be used without testing.

**Notation:**

Authority: Health and Safety Code Section 18959.5

Reference: Health and Safety Code Sections 18959 and 18959.5

**Item 19**

**8-805.2.2 Independent Wythe Stone Masonry.** Stone masonry with independent face wythes may be treated as solid brick masonry as described in Section 8-805.1 and the ~~UCBC~~ ICEBC, provided representative testing and inspection verify that the core is essentially solid in the masonry wall and that steel ties are epoxied in drilled holes between outer stone wythes at floors, roof and at not-to-exceed 4 feet (1219 mm) on center in each direction, between floors and roof. ~~A reinforced concrete bond beam or equivalent structural element shall be provided at the top of all stone masonry walls. A reinforcing element shall exist or be provided at or near the top of all stone masonry Walls.~~

**Rationale for change:**

SHBSB proposes to remove the last sentence of Section 8-805.2.2 based on a stakeholder comment recommending its removal because it is arbitrary and overly-prescriptive. The Board agreed and added a clarifying sentence in its place, adding the provision of “strengthening element.” The last sentence added recognizes

that there may or may not be a reinforcing element. The determination of the placement of a reinforcing element will rely on engineering judgment.

**Notation:**

Authority: Health and Safety Code Section 18959.5

Reference: Health and Safety Code Sections 18959 and 18959.5

**Item 20**

**8-805.2.3 Testing of Stone Masonry.** Testing of stone masonry shall be similar to ~~UBC Standard 21-6, as referenced in the 1997 UBC the 200610 CEBC requirements for brick masonry,~~ except that representative stones which are not interlocked shall be pulled outward from the wall and shear area appropriately calculated after the test.

**Rationale for Change:**

SHBSB proposes to not to accept the comments relating to the testing of stone masonry. The proposal is to change the reference to the current edition of the International Existing Building Code, and therefore the CEBC, and does not address subject of the code text – the edition of the CEBC.

**Notation:**

Authority: Health and Safety Code Section 18959.5

Reference: Health and Safety Code Sections 18959 and 18959.5

**Item 21**

**8-806.1 General.** Unburned clay masonry may be constructed, reconstructed, stabilized, or rehabilitated subject to this chapter. ~~When undertaking a mandatory or voluntary retrofit, the following provisions should be considered.~~ Alternate approaches which provide an equivalent or greater level of safety may be used, subject to the concurrence of the enforcing agency.

**Rationale for change:**

SHBSB proposes to accept the public comment to delete the added section.

**Notation:**

Authority: Health and Safety Code Section 18959.5

Reference: Health and Safety Code Sections 18959 and 18959.5

**Item 22**

**8-806.2 Moisture Protection.** Provisions shall be ~~made in-place~~ to protect adobe structures from ~~moisture and deterioration due to moisture penetration.~~ The ~~un-reinforced~~ Adobe shall be maintained in reasonably good condition. Particular attention shall be given to moisture content of adobe walls. Un-maintained ~~or un-stabilized~~ walls or ruins shall be evaluated for safety based on their condition and stability. Additional ~~safety protection~~ measures may be ~~required~~ appropriate subject to the concurrence of the enforcing agency.

**Rationale for change:**

SHBSB proposes to amend Section 8-806.2 based on a stakeholder comment recommending the deletion of the word “or” after the word “un-maintained” in the fourth line of the section. This is a non-substantive grammatical correction.

**Notation:**

Authority: Health and Safety Code Section 18959.5

Reference: Health and Safety Code Sections 18959 and 18959.5

**Item 23**

**8-806.3 Requirements-Height to Thickness Ratio:** Unreinforced new or existing adobe walls that meet these ~~requirements~~ criteria need not be evaluated for out of plane failure. ~~Existing sod or rammed earth walls~~

~~shall be considered similar to the extent these provisions apply.~~ Where existing dimensions do not meet these conditions, additional strengthening measures, such as a bond beam, may be ~~required~~ appropriate. Existing sod or rammed earth walls shall be considered similar to the extent these provisions apply.

**Rationale for change:**

SHBSB proposes to amend Section 8-806.3 based on a stakeholder comment recommending the reversal of the H/T ratio. The Board did not accept the comment regarding the bond beam as commentary.

**Notation:**

Authority: Health and Safety Code Section 18959.5

Reference: Health and Safety Code Sections 18959 and 18959.5

**Item 24**

**3- 8-806.4 Non Load-bearing Adobe.** Non-load-bearing adobe partitions and gable end walls shall be evaluated for stability and anchored against out-of-plane failure if necessary.

SHBSB received a comment inquiring if there was a minimum thickness for interior adobe walls. This comment is not related to the amendment proposed. No response necessary.

**Item 25**

**8-806.5 Bond Beam.** Where ~~required~~ provided, Aa bond beam or equivalent structural element shall be ~~provided~~ located at the top of all adobe walls, and at the second floor for two-story buildings or structures at the second floor. The size and configuration of the bond beam structural element shall be designed sufficient in each case to meet the requirements of the existing conditions and provide an effective brace for the wall, to tie the building together and to connect the wall to the floor or roof.

**Rationale for change:**

SHBSB proposes to amend Section 8-806.5 based on a stakeholder comment recommending changing the term “required” to “provided” in the first sentence.” The board accepted the comment that “Where required” was redundant.

**Notation:**

Authority: Health and Safety Code Section 18959.5

Reference: Health and Safety Code Sections 18959 and 18959.5

**Item 27**

**8-806.5 8-806.7 Shear Values.** Existing adobe may be allowed a maximum ultimate value strength level of four twelve pounds per square inch (27.6 82.7 kPa) for shear, ~~with no increase for lateral forces.~~

**Rationale for change:**

SHBSB proposes to amend Section 8-806.7 based on a stakeholder comment recommending that the term “ultimate strength” be replaced by “strength level.” The text reflects that change; this is consistent with previous changes made by the Board based on similar comments.

**Notation:**

Authority: Health and Safety Code Section 18959.5

Reference: Health and Safety Code Sections 18959 and 18959.5

**Item 31**

**TABLE 8-8-A—ALLOWABLE STRENGTH VALUES FOR EXISTING MATERIALS**

EXISTING MATERIALS OR CONFIGURATIONS OF MATERIALS <sup>1</sup>	ALLOWABLE VALUES Strength Level Capacity
	x14.594 for N/m
1. Horizontal diaphragms <sup>2</sup> 1.1 Roofs with straight sheathing and roofing applied directly to the sheathing 1.2 Roofs with diagonal sheathing and roofing applied directly to the sheathing 1.3 Floors with straight tongue-and-groove sheathing 1.4 Floors with straight sheathing and finished wood flooring with board edges offset or perpendicular 1.5 Floors with diagonal sheathing and finished	400 <del>300</del> lbs. Per foot for seismic shear 250 <del>750</del> lbs. Per foot for seismic shear 400 <del>300</del> lbs. Per foot for seismic shear 500 <del>1500</del> lbs. Per foot for seismic shear 600 <del>1800</del> lbs. Per foot for seismic shear
2. Crosswalls <sup>2,3</sup> 2.1 Plaster on wood or metal lath 2.2 Plaster on gypsum lath 2.3 Gypsum wallboard, unblocked edges 2.4 Gypsum wallboard, blocked edges	Per side: 200 <del>600</del> lbs. Per foot for seismic shear 475 <del>550</del> lbs. Per foot for seismic shear 75 <del>200</del> lbs. Per foot for seismic shear 425 <del>400</del> lbs. Per foot for seismic shear
3. Existing footings, wood framing, structural steel and reinforcing steel 3.1 Plain concrete footings 3.2 Douglas fir wood 3.2 Reinforcing steel 3.4 Structural steel	$f'_c = 1,500$ psi (10.34 MPa) unless otherwise shown by tests <sup>4</sup> Allowable stress same as D.F. No. 1 <sup>4</sup> $f_t = 48,000$ <del>40,000</del> lbs. Per square inch (124.1 <del>MN</del> /mm <sup>2</sup> ) maximum $f_t = 20,000$ <del>33,000</del> lbs. Per square inch (137.9 N/mm <sup>2</sup> ) maximum <sup>4</sup>

<sup>1</sup>Material must be sound and in good condition.

<sup>2</sup>A one-third increase in allowable stress is not allowed.

<sup>3,2</sup>Shear values of these materials may be combined, except the total combined value shall not exceed 300 ~~900~~ pounds per foot (4380 ~~13,140~~ N/m).

<sup>4,2</sup>Stresses given may be increased for combinations of loads as specified in the regular code.

**Rationale for change:**

SHBSB proposes to amend Table 8-8-A based on a stakeholder comment recommending the heading in the second be changed to read Strength Level Capacity. The SHBSB believes that the provisions in Table 8-8-A are appropriate based on the values shown. The comment about the values being inconsistent with Section 8-706.1 does not recognize the proposed modifications to that section. The SHBSB believes that the performance objectives are adequately addressed in Section 8-701.

**Notation:**

Authority: Health and Safety Code Section 18959.5

Reference: Health and Safety Code Sections 18959 and 18959.5

**Item 32**

**TABLE 8-8-B—ALLOWABLE STRENGTH VALUES OF NEW MATERIALS USED IN CONJUNCTION WITH EXISTING CONSTRUCTION**

NEW MATERIALS OR CONFIGURATIONS OF MATERIALS	ALLOWABLE VALUES <sup>4</sup> STRENGTH LEVEL CAPACITY <sup>1</sup>
<p>1. Horizontal diaphragms<sup>2</sup></p> <p>Plywood sheathing nailed directly over existing straight sheathing with ends of plywood sheets bearing on joists or rafters and edges of plywood located on center of individual sheathing boards</p> <p>Plywood sheathing nailed directly over existing diagonal sheathing with ends of plywood sheets bearing on joists or rafters</p> <p>1.3 Plywood sheathing nailed directly over existing straight or diagonal sheathing with ends of plywood sheets bearing on joists or rafters with edges of plywood located over new blocking and nailed to provide a minimum nail penetration into framing and blocking of 1_ inches (41 mm)</p>	<p>225 lbs. Per foot (3283 N/m)</p> <p>375 lbs. Per foot (5473 N/m)</p> <p>75 percent of the values specified in the regular code</p>
<p>1. Horizontal Diaphragms<sup>2</sup></p> <p>1.1 15/32 inch minimum plywood sheathing fastened directly over existing straight sheathing with ends of plywood sheets bearing on joists or rafters and edges of plywood located on center of individual sheathing boards and fastened with minimum #6x 1 ¼ that the screw inch wood screws or nails with helical threads 0.13 inch min. diameter and 1 ¼ inch min. length at 4 inch centers all panel edges and 12 inch centers each way in field.</p> <p>1.2 Same plywood and attachments as 1.1 fastened directly over existing diagonal sheathing.</p> <p>1.3 3/8 inch plywood sheathing fastened directly over existing straight or diagonal sheathing with ends and edges on centers of individual sheathing boards and fastened with #6 wood screws or nails with helical threads 0.13 inch minimum diameter and 1 ¼ inch min. length at 6 inch centers tall panel edges and 12 inch centers each way in field.</p>	<p>1,500 lbs. Per foot</p> <p>1,800 lbs. Per foot</p> <p>900 lbs. Per foot</p>
<p>2. Shear walls: (general procedure)</p> <p>Plywood sheathing applied directly over wood studs. No value shall be given to plywood applied over existing plaster or wood sheathing</p>	<p>100 percent of the value specified in the regular code for shear walls</p>
<p>3. Crosswalls: (special procedure only)</p> <p>Plywood sheathing applied directly over wood studs. No value shall be given to plywood applied over existing plaster or wood sheathing</p> <p>Drywall or plaster applied directly over wood studs</p> <p>Drywall or plaster applied to sheathing over existing wood studs</p>	<p>133 percent of the value specified in the regular code for shear walls</p> <p>100 percent of the values in the regular code.</p> <p>50 percent of the values specified in the regular code, reduced as noted.<sup>3</sup> (UBC Table 25-I, Footnote 4)</p>
<p>4. Tension bolts</p> <p>a. Bolts extending entirely through unreinforced masonry walls secured with bearing plates on far side of a three-wythe-minimum wall with at least 30 square inches (19 350 mm<sup>2</sup>) of area<sup>4,5</sup></p> <p>b. Bolts All thread rod extending to the exterior face of the wall with a 2½-inch (63.5 mm) round plate under the head and drilled at an angle of 22½ degrees to the horizontal, installed as specified for shear bolts<sup>4,5,7</sup> in adhesive<sup>9</sup></p>	<p>4,800 5400 lbs. (8006 16,200 N) per bolt<sup>6</sup></p> <p>900 2700 lbs. (4003 12,009 N) per bolt for two-wythe walls<sup>6</sup></p> <p>4,200 3600 lbs. (5338 10,008 N) per bolt</p>
<p>5. Shear bolts</p> <p>Bolts embedded a minimum of 8 inches (203 mm) into unreinforced masonry walls and centered in a 2½-inch-diameter (63.5 mm) hole filled with drypack or non-shrink grout. Through bolts with first 8 inches (203 mm) as noted above and embedded bolts all thread rod as noted in Item 4.2b<sup>5,7,9</sup></p>	<p>½ inch (12.7 mm) diameter = 350 1050 lbs. (1557 4671 N)<sup>6</sup></p> <p>⅝ inch (15.9 mm) diameter = 500 1500 lbs. (2224 6672 N)<sup>6</sup></p> <p>¾ inch (19 mm) diameter = 750 2250 lbs. (3336 16,014 N)<sup>6</sup></p>
<p>6. Infilled walls</p> <p>Reinforced masonry infilled openings in existing unreinforced masonry walls. Provide keys or dowels to match reinforcing</p>	<p>Same as values specified for unreinforced masonry walls.</p>
<p>7. Reinforced masonry</p> <p>Masonry piers and walls reinforced per the regular code</p>	<p>Same as values specified in the regular code<sup>8</sup>.</p>
<p>8. Reinforced concrete</p> <p>Concrete footings, walls and piers reinforced as specified in the regular code and designed for tributary loads</p>	<p>Same values as specified in the regular code<sup>8</sup>.</p>

<sup>1</sup>A one-third increase in allowable stress is not allowed, except as noted. Values are for strength level loads as defined in regular code standards.

<sup>2</sup>Values and limitations are for nailed plywood. Higher values may be used for other fastening systems such as

wood screws or staples when approved by the enforcing authority. Values may be adjusted for other fasteners when approved by the enforcing authority.

<sup>3</sup>In addition to existing sheathing value.

<sup>4</sup>Bolts to be ½-inch (12.7 mm) minimum diameter.

<sup>5</sup>~~Drilling for bolts and dowels shall be done with an electric rotary drill. Impact tools shall not be used for drilling holes or tightening anchors and shear bolt nuts. Other bolt sizes, values and installation methods may be used provided a testing program is conducted in accordance with regular code standards. Bolt spacing shall not exceed 6 feet. (1830 mm) on center and shall not be less than 12 inches (305 mm) on center.~~

<sup>6</sup>~~Other masonry based on tests or other substantiated data.~~

<sup>7</sup>Embedded bolts to be tested as specified in regular code standards.

<sup>8</sup>Stresses given may be increased for combinations of loads as specified in the regular code.

<sup>9</sup>~~Adhesives shall be approved by the enforcing agency and installed in accordance with the manufacturer's recommendations. All drilling dust shall be removed from drilled holes prior to installation.~~

**Rationale for change:**

SHBSB proposes to amend the heading in right column of Table 8-8-B to read Strength Level Capacity for consistency based on a stakeholder. SHBSB accepts a comment recommending that the screw size #6 in item 1.1 under Diaphragms be changed to #8. The SHBSB is removing a portion of the second line in item 1.1 to respond to a comment that addresses an issue with the edges of the plywood and the transfer of shear to the edge members.

**Notation:**

Authority: Health and Safety Code Section 18959.5

Reference: Health and Safety Code Sections 18959 and 18959.5