

**INITIAL STATEMENT OF REASONS
OF THE
OFFICE OF STATEWIDE HEALTH PLANNING AND DEVELOPMENT**

**REGARDING PROPOSED CHANGES TO THE
CALIFORNIA ADMINISTRATIVE CODE
AND
CALIFORNIA BUILDING CODE
CALIFORNIA CODE OF REGULATIONS, TITLE 24, PARTS 1 & 2**

The Administrative Procedure Act (APA) requires that an Initial Statement of Reasons (ISOR) be available to the public upon request when rulemaking action is being undertaken. The following information required by the APA pertains to this particular rulemaking action:

STATEMENT OF SPECIFIC PURPOSE, PROBLEM, RATIONALE and BENEFITS:

The purpose of this proposed action is to adopt the 2013 California Administrative Code (2013 CAC) and 2013 California Building Code (2013 CBC) based on new information since the adoption of the 2010 CAC and 2010 CBC.

**Title 24, Part 1
Chapter 6 - Seismic Evaluation Procedures for Hospital Buildings**

Section 2.7 – Editorial, for consistency with the 2013 CBC.

Table 11.1 – This section is revised to limit the scope to what need to done to achieve Nonstructural Performance Category 5. How the requirements shall be satisfied is addressed in the California Plumbing Code and California Electrical Code. Provisions that duplicate requirements in the California Plumbing Code are deleted.

**Title 24, Part 1
Chapter 7 – Safety Standards for Healthcare Facilities**

Section 7-117 – Editorial, for consistency with the 2013 CBC.

**Title 24, Part 2, Volume 2
Chapter 1 – Scope and Administration**

Sections 102.1.1 & 107.4 – Redundant pointers are deleted.

Section 104.11.4 – Alternative system requirements in ASCE 7 is incorporated by reference. Additional requirements are provided for the earthquake monitoring instrumentation so that consistent data is obtained and the instruments are not obstructed for the purposes of service or repair. This change is required for consistency with Appendix L.

**Title 24, Part 2, Volume 2
Chapter 2 – Definition**

Section 202 – Definitions that affect multiple chapters are relocated from other chapters to Chapter 2 for consistency with new format of the CBC 2013. Definition of active equipment/component, rugged equipment, and significant loss of function which is currently in Code Application Notice (CAN) 2-1708A.5 and needed for special seismic certification requirements in Chapter 17A is codified.

Title 24, Part 2, Volume 2
Chapter 16A – Structural Design

Section 1603A.2 – This section will require the Registered Design Professional (RDP) to submit a description of project for which site data reports are submitted, so that relevance and adequacy of the report can be verified as required by the California Health and Safety Code Section 129770.

Section 1603A.3 – This section is added to comply with the California Health and Safety Code Section 129765.

Table 1604A.3 – Editorial, limit is shown explicitly instead of referring to another section.

Section 1604A.3.7 – Diaphragm span to width ratio requirement is added explicitly instead of referring to ICC-ES AC 43 for easy reference.

Section 1604A.8.2 – This section has been modified to retain the 280 plf minimum strength level force for concrete and masonry wall anchorage design to diaphragms as given in ASCE 7-05. A similar provision has been in the CBC since 1979, which was added based on post-earthquake observation.

Section 1609A.1.3 – Editorial to convert nominal design wind speed factors to factors based on ultimate design wind speed.

Section 1613A.4.1 – Editorial to update section reference to AISC 341-10.

Sections 1615A.1.7, 1615A.1.9, 1615A.1.13, 1615A.1.14, 1615A.1.15, 1615A.1.22, 1615A.1.25, 1615A.1.26, 1615A.1.35, & 1615A.1.39 (of CBC 2010) – These existing sections are deleted because the model building code had been revised to match with OSHPD amendments.

Section 1616A.1.1 – The requirements for a Structural Design Criteria has been revised to incorporate wind loading based upon wind tunnel testing and moved to Chapter 1 of ASCE 7 since it no longer is strictly seismic requirements.

Section 1616A.1.2 – This amendment will ensure ductile detailing for the central utility plants and other unoccupied buildings required for continuous operation of hospitals.

Section 1616A.1.4 –The Eccentric Braced Frame (EBF) and Buckling Restrained Braced Frame (BRBF) systems in AISC 341-10 and ASCE 7-10 are no longer specifically categorized as to the type of beam-to-column connections. Therefore, these ASCE 7-05 system names are removed. The Cold-formed steel-Bolted Moment Frame system is not permitted in buildings since it is not included in AISC 341-10, and it was developed and tested specifically for non-building structures, where the developed lateral strength at the more stringent building drift limit levels was not considered and, as a result, is overestimated. In addition, permitted configuration for moment frame goes beyond the test parameters to be considered applicable.

Section 1616A.1.5 – Requirements for the R , C_d and Ω_o values in vertical combinations provisions in ASCE 7-05 Section 12.2.3.1 are retained, since ASCE 7-10 Section 12.2.3.1 essentially permits a two stage analysis without the requirements of Section 12.2.3.2. ASCE 7-10 Section 12.2.3.1 will encourage the use of different and possibly deformation incompatible seismic force resisting systems near the mid-height of the building.

Section 1616A.1.12 – Maximum mapped short period acceleration (S_s) in California is about 3.73 and site specific short period acceleration are sometimes even higher. Permitting an $S_s = 1.5$, will mean a reduction of more than 60% from the mapped value. Most of these regular structures are designed with a Redundancy Factor, $\rho = 1.0$, compared with 1.3 for most irregular structures. This mean irregular

structure may potentially be designed for a force of about 3.2 times that of a regular structure at the same site. Historical records of building performance don't to justify such a large advantage for the regular structures. A 25% maximum reduction in short period acceleration seems to be more appropriate. Recent ATC – 58 project analysis suggested lower R values for low rise buildings, which also justify the proposed amendment.

Section 1616A.1.14 – This 2010 CBC section has been revised to incorporate ASCE 7-10 Section 12.10.2.1, which is similar to the previous amendment, but the upper bound limit is modified to remain the same as in the 2010 CBC amendment

Section 1616A.1.15 – Amendments retains building separation requirements identical to what is required by the CBC 2010, which ensures that buildings with same forces and stiffness will have equal building separation. This revision is intended to assure the Immediate Occupancy and Operational Performance Levels for hospital buildings.

Sections 1616A.1.16, 1616A.1.17,– Editorial

Section 1616A.1.18 . The exemption for distributed systems have been modified, so that where these systems are subject differential displacements within structures or cross seismic separation joints between adjacent buildings, adequate flexibility to accommodate the movements without failure is provided.

Section 1616A.1.19 – Portions of this 2010 CBC amendment are removed since ASCE 7-10 Section 13.4.2 contains provisions similar to those removed. Use of screw anchors are limited to interior conditions based on installation problems observed in the field. Thread of screw anchors do not hold and keep spinning & do not permit a proper installation.

Section 1616A.1.23 – Portions of this 2010 CBC amendment are removed since ASCE 7-10 Section 13.6.5.6 contains provisions similar to those removed. The exemptions for small raceways and conduit have been modified, so that where these systems are subject differential displacements within structures or cross seismic separation joints between adjacent buildings, adequate flexibility to accommodate the movements without failure is provided.

Section 1616A.1.24 – Portions of this 2010 CBC amendment are removed since ASCE 7-10 Section 13.6.7 contains provisions similar to those removed. The exemptions for small ducts have been modified, so that where these systems are subject differential displacements within structures or cross seismic separation joints between adjacent buildings, adequate flexibility to accommodate the movements without failure is provided.

Section 1616A.1.26 – Portions of this 2010 CBC amendment are removed since ASCE 7-10 Section 13.6.8.2 contains provisions similar to those removed. The exemptions for small pipes have been modified, so that where these systems are subject differential displacements within structures or cross seismic separation joints between adjacent buildings, adequate flexibility to accommodate the movements without failure is provided.

Section 1616A.1.27 – Portions of this 2010 CBC amendment are removed since ASCE 7-10 Sections 13.6.8.3, 13.6.9 and 13.6.10 contain provisions similar to those removed.

Section 1616A.1.29 Portions of this 2010 CBC amendment are removed since ASCE 7-10 Section 16.1.4 is similar to the previous amendment and this section also modified to be consistent with that amendment in ASCE 7 Section 12.9.4.

Section 1616A.1.40 – The criteria for when members can be modeled as linear in the nonlinear response history procedure is further clarified as the maximum response from any single ground motion.

Otherwise, the average response of member deformations is underestimated when using multiple ground motions due to a lack of appropriate nonlinear member modeling.

Section 1616A.1.41 - Additional requirements are provided for the earthquake monitoring instrumentation so that consistent data is obtained and the instruments are not obstructed for the purposes of service or repair. This change is necessary for consistency with Appendix L.

Section 1616A.1.42 - This section is revised to require that new general acute care hospitals and new buildings required for general acute services are compliant with Nonstructural Performance Category 5 (NPC 5) as required by the California Health and Safety Code Sections 129680 and 13005.

Title 24, Part 2, Volume 2

Chapter 17A - Special Inspections and Tests

Section 1701A.4 – Editorial.

Section 1702A.1 – Definition of continuous and periodic special inspection from the CBC 2010 are retained for consistency the CAC 2013.

Section 1702A.2 – Existing definition in the CBC 2010 is retained for consistency with the CAC 2013.

Sections 1704A.3.2 & 1704A.4 – Editorial changes to make terminology consistent throughout the code.

Section 1704A.5 – This section replaces 2010 CBC Sections 1710A.2 and 1710A.3, which are equivalent.

Section 1705A.2.1 – Inspection requirements provided in the AISC 360 Chapter N, which is new to the code, is revised to maintain inspection requirements consistent with the CBC 2010 requirements.

Section 1705A.3 – The Exceptions to required Special Inspections in this section are removed since the minimum concrete compressive strength for new hospital buildings is 3000 psi and the referenced Tables in this section in Chapter 18A are deleted.

Table 1705A.3 – The proper installation of adhesive anchors in horizontal and overhead conditions have been known to be difficult. To preclude potential failures, as illustrated by a recent catastrophic failure associated with the use of these anchors in the overhead position, an ACI/CRSI Certified Adhesive Anchor Installer is required for the installation in accordance with ACI 318-11 Section D.9.2.2.

Sections 1705A.4 & 1705A.4.1 – Testing and inspection requirements for post-installed anchors in Masonry is clarified.

Sections 1705A.5, 1705A.6.1, 1705A.11, and 1705A.11.4 – Editorial.

Section 1705A.11.5 – Periodic Special Inspection of ceilings are added to the other non-structural components which require Special Inspection to assure proper installation of seismic bracing, grid runner continuity and perimeter edge detailing in order to maintain ceiling functionality under earthquake forces and deformations.

Section 1705A.12 – Editorial.

Section 1705A.12.3 – FM 1950 is adopted for seismic sway brace testing to codify current OSHPD practice, consistent with requirements of NFPA 13.

Section 1705A.12.4 – This section is separated from Section 1705A.12.3 for manufacturer's certification for clarity. Certain equipment which are considered to be critical for hospital operations have been added to the list of equipment requiring special seismic certification.

Title 24, Part 2, Volume 2

Chapter 18 – Soils and Foundations

Section 1803.7 – The geologic and earthquake engineering reports provisions have been consolidated

into the provisions for one geohazard report in order to simplify geotechnical and ground motion related provisions.

Title 24, Part 2, Volume 2
Chapter 18A – Soils and Foundations

Section 1803A.2 – Editorial to clarify that existing geotechnical reports that satisfying the current code requirements are acceptable.

Section 1803A.6 - All requirements beyond the model code geotechnical report and basic geohazard reports are deleted. Revisions to model code Section 1803A.5.1 through 1803A.5.12 adequately address the deleted amendments.

Section 1803A.6.2 – This 2010 CBC provision's site specific probabilistic and deterministic site hazard requirements have been removed since similar provisions are now contained under ASCE 7-10 Section 21.2.

Section 1803A.7 – Editorial, for the purposes of renaming the engineering geologic reports.

Section 1807A.2.2 – The lateral soil pressures determined by the geotechnical investigation are revised to incorporate a minimum value of 80% of the presumptive lateral soil pressures determined by Section 1610A. This minimum value concept is similar to that used for other site-specific procedures, such as for ground motion. The site-specific value should not be significantly less than the code table based values for the typical condition.

Section 1810A.3.3.1.2 – This section is revised to add the Procedure G: Cyclic Loading in ASTM D 1143 to the pile compression testing. Procedure G will account for the cyclic loading that is inherent with piles under the Design Earthquake loading in Seismic Design Categories D through F.

Section 1810A.3.3.1.5 – This section is revised to add the Cyclic Loading procedure in ASTM D 3689 to the pile tension testing. The Cyclic Loading procedure will account for the cyclic loading that is inherent with piles under the Design Earthquake loading in Seismic Design Categories D through F.

Section 1810A.3.3.2 – This section is revised to add ASTM D 3966 - Standard Test Method for Piles Under Lateral Loads which has been omitted by the model building code. The Cyclic Loading procedure in ASTM D 3966 is also added to the pile lateral load testing. The Cyclic Loading procedure will account for the cyclic loading that is inherent with piles under the Design Earthquake loading in Seismic Design Categories D through F.

Section 1810A.3.9.4.2.1 – Editorial, the transverse confinement reinforcing for the pile is located at the bottom of the pile cap.

Section 1810A.3.9.4.2.2 – Editorial, the transverse confinement reinforcing for the pile is located at the bottom of the pile cap.

Title 24, Part 2, Volume 2
Chapter 19 - Concrete

Section 1908.1.1 – The use of power actuated fasteners is permitted for only non-structural components of minimal concern consistent with scope of ICC-ES AC 156, since acceptance criteria does not include dynamic testing.

Section 1909.1.1 and 1909.1.2 – Exceptions are deleted since requirements are now addressed in the

model code.

Section 1909.2.5 – This section is modified to clarify when torque testing of post installed anchors is permitted versus tension testing of the anchors.

Title 24, Part 2, Volume 2
Chapter 19A - Concrete

Section 1903A.3 – Insulating Concrete Form Systems are typically used for residential and light commercial construction and should not be used for non-combustible (mostly Type 1) construction in OSHPD 1 and 4 hospital buildings. Also, there are concerns about using exposed form on air quality in a hospital environment.

The forms are interlocking modular units that are dry-stacked (without mortar) and filled with concrete. The forms lock together somewhat like Lego bricks and serve to create a form for the structural walls or floors of a building.

ICFs are currently manufactured from any of the following materials:

- Polystyrene foam (expanded or extruded — most common)
- Polyurethane foam (including soy-based)
- Cement-bonded wood fiber
- Cement-bonded polystyrene beads

Concrete is pumped into the cavity to form the structural element of the walls.

There is no systematic study to evaluate performance of ICFs when subjected to seismic forces.

Sections 1905.1.1, 1905.1.2, 1905.1.4, 1905.1.5, 1905.1.6, 1905.1.7, 1905.1.8 and 1905.1.10 (of IBC 2012) – These sections from IBC 2012 are deleted because ACI 318 requirements are almost identical to the deleted sections due to adoption of ACI 318-11 after the adoption of the IBC 2012.

Section 1905A.1.15 – The existing amendment for span-to-depth ratios for pre-stressed concrete members is retained with editorial changes to account for the deleted 2010 CBC Section 1908A.1.21 reference.

Section 1905A.1.21 – The proposed change revises Chapter 19 of the 2012 IBC so that it is consistent with ACI 318-11. Although 2012 IBC Chapter 35 references ACI 318-11, the text in 2012 IBC Chapter 19 erroneously reflects modifications to ACI 318-08. This code change is necessary to keep the 2013 CBC consistent with ACI 318-11.

- a. Sections D.3.3.4, D.3.3.5, D.3.3.6 and D.3.3.7, as added by 2012 IBC Section 1905.1.9, are deleted because they are not consistent with ACI 318-11 Appendix D.
- b. In the exception to ACI 318-11 Section D.3.3.4.2, “need not satisfy” is changed to “shall be deemed to satisfy” to convey the message: it is not that the relevant provisions are not necessary, but that the design forces already incorporate the requirement.
- c. The second sentence of D.3.3.4.3 (d) is revised to make it consistent with the language used in D.3.3.4.3 (b) and D.3.3.4.3 (c). The sentence currently reads: “The anchor design tensile strength shall satisfy the tensile strength requirements of D.4.1.1.” This is changed to: “The anchor design tensile strength shall be calculated from D.3.3.4.4.”
- d. Exceptions for anchors in wood sill plates and cold-formed steel tracks are revised for better clarity. In addition to the concrete breakout in shear, anchor pryout in shear is also included in the

exception now. For in-plane shear in a sill plate or a cold-formed steel track, anchor pryout is not a concern.

Section 1908A.1.20 (CBC 2010) – This 2010 CBC section is deleted since the content is similar to ACI 318 Sections 18.3, 18.4, and 21.5.2.5.

Section 1908A.1.21 (CBC 2010) – This 2010 CBC section is deleted since the content is similar to ACI 318 Sections 18.12 and 18.14.

1908A.1.1 – Editorial changes to remove reference to International Code Council – Evaluation Service Report (ICC-ESR).

Section 1913A.7 – This section is modified to remove reference to ICC-ESR and to clarify when torque testing of post installed anchors is permitted versus tension testing of the anchors.

Title 24, Part 2, Volume 2 Chapter 21A - Masonry

Section 2101.1.3 – Direct design of Masonry is added to the list of prohibited design method list for consistency with Section 2101A.2.7.

Section 2101A.2 – Section revised for consistency with Section 2101A.1.3 and 2101A.2.7.

Section 2101A.2.6 - Current IBC language incorrectly suggests that veneer could be designed and installed bypassing Chapter 14. Deleting reference to TMS 402 Chapter 6 requires user to comply with the requirements of Chapter 14, then the user is required to go to TMS 402, Chapter 6 for any additional veneer requirements. This deletion is adopted for the 2015 IBC.

Section 2101.2.7 – TMS 403 is not permitted, since it is not consistent with the rest of CBC 2013 provisions. TMS 403 is based on ASCE 7-05 and TMS 402-08, which are superseded by newer standard in the CBC 2013.

Section 2103A.9 – Deleted amendment is picked-up by TMS 402-10 Section 1.18.4.4.2.2.

Section 2103A.13.2 – Deleted part of amendment is covered by ASTM C476, Table 1 and ASTM C476 is referenced by TMS 602, Article 2.2 A.

Section 2104A.1.7 (CBC 2010) – This section, which is a pointer to Section 2405A.5, is deleted because Section 2104.5 is included in the scope covered by Section 2104A.1.

Section 2104A.5 & 2104A.6 – Editorial. Sections are reorganized for ease of use in accordance with recommendation by Masonry Industry Association (MIA).

Section 2104A.5.1 - TMS 402 Section 1.17.1 and TMS 602 Section 3.4D.3 now permit bolts to be installed in tight fitting holes in the face shell. Installation of headed bolts would need to be done from inside the shell and for 8” and smaller block, this would be difficult to get any decent embedment. Also, there is no definition of “tight fitting”. Minimum grout requirement will ensure installation to meet the intent of the code.

Section 2105A.2.1 (Exception) – Requirement for mortar and grout tests during construction is deleted for consistency with Section 2105A.2.2.1.4. Constructed masonry is subject to both prism and core tests, hence additional mortar and grout tests are considered redundant.

Section 2107A.1.6 (CBC 2010) – This section is deleted because TMS 402 Sections 1.13.1.3 is more restrictive.

Section 2107A.2.1 - Amendment is necessary to bring consistency in lap splice length for allowable stress design in the CBC 2013 Section 2107A.2.1 and strength design in Section 2108A.2.

Section 2107A.6 (CBC 2010) – This section is deleted because TMS 402 addressed the edge distance and bolt spacing in a comprehensive way.

Section 2115A.1 – Section is revised for consistency with TMS Sections 7.2.2, 7.3.3, and 7.7.

Title 24, Part 2, Volume 2

Chapter 22A - Steel

Sections 2204A.1.1, 2204A.1.2, and 2204A.1.3 – These sections are deleted since they are picked-up by model code.

Sections 2205A.4.1.2, 2205A.4.1.3, 2205A.4.1.4, 2205A.4.1.7 – These existing amendments are deleted since the steel OMF, IMF, OCBF and STMF systems will be permitted in SDC D, E, and F as non-building structure systems in Chapter 15 of ASCE 7-10.

Section 2205A.4.2.2 - This existing amendment for steel moment resisting frame connection definition of “Rapid Strength Deterioration” is removed since the minimum strength of $0.80 M_p$ at the qualifying interstory drift angle as required by AISC 341-10 for the IMF and SMF is sufficient.

Section 2205A.4.3 - The existing amendment for buckling restrained brace provisions for similitude between the prototype and test specimen brace is removed due to similar provisions in AISC 341-10.

Section 2205A.3.4 – The square HSS brace section is clarified to be a subset of rectangular HSS sections and subject to the requirements in this provision. The inherent deficiency and poor performance of square or rectangular HSS brace sections under plastic hinging mechanisms due to axial seismic deformations has not been fully rectified by the provisions of AISC 341-10.

Section 2205A.3.7 - The steel moment resisting frame connection acceptance criteria for “Rapid Strength Deterioration” has been removed since the minimum strength of $0.80 M_p$ at the qualifying interstory drift angle as required by AISC 341-10 for the IMF and SMF is sufficient and more restrictive.

Section 2205A.4 – The ConXtech ConXL bolted moment connection contained in AISC 358-10 is added to the Exception. The ConXtech ConXL bolted moment connection has been successfully cyclic tested specifically for the purposes of meeting the acceptance criteria as set forth in Section 2205A.3.

Section 2206A.2 – Amendment is relocated from section 2205A.3.1. The Exception has been added to permit the use of the ConXtech ConXL bolted moment connection as given in Chapter 10 of AISC 358-10 Supplement No. 1 with the amendments shown. The amendments are based upon the cyclic test specimens used to meet the acceptance criteria as set forth in Section 2205A.3.

Section 2208A.2 – Deleted for consistency with ASCE 7-10 Supplement # 1 and ASCE 19-10.

Section 2210A.1.1.3 (CBC 2010 Section 2209.3) – ANSI/SDI C-2012 is adopted for consistency with IBC 2015. SDI C will replace ICC-ES AC 43, which is not an ANSI approved standard, in the CBC 2010 Section 2209A.3

Sections 2213A.1 & 2213A.2 – Editorial changes to clarify amendments and make them more specific. This will limit the scope of field tests, consistent with current practice.

Title 24, Part 2, Volume 2
Chapter 23 - Wood

Section 2302.1 - IBC errata.

Section 2305.1.3 – Section is relocated to item # 6 in Section 2305A.1.2.

Section 2305.1.4, 2306.4, and 2306.7 – Deleted amendments are picked-up by model code.

Title 24, Part 2, Volume 2
Chapter 24 – Glass and Glazing

Section 2410 – Provisions for use of Structural Sealant Glazing (SSG), which is currently not addressed in the model building code, is added so that they will not be considered as an alternative system requiring structural design criteria.

Title 24, Part 2, Volume 2
Chapter 25 – Gypsum Board and Plaster

Section 2510.7.1 – Section is deleted, since requirement is picked-up in model code Table 2507.2.

Title 24, Part 2, Volume 2
Chapter 33 – Safeguard During Construction

Sections 3307.2 and 3307.3 – These amendments are deleted since requirements in Sections 3307 and J106.2 adequately addresses this subject.

Title 24, Part 2, Volume 2
Chapter 34 - Existing Structures

Section 3401.7 – Editorial.

Section 3401.7.2 – Ground motion requirements are revised to incorporate risk targeted ground motion consistent with ASCE 7.

Title 24, Part 2, Volume 2
Chapter 34A - Existing Structures

Section 3404A.5, 3412A.1, & 3412A.2 – Editorial.

Sections 3405A.1 & 3405A.3.1 – Redundant exceptions, which apply to buildings outside the scope of Chapter 34A, are deleted.

Section 3412A.2 – Editorial.

Section 3413A .1.2 – Ground motion requirements are revised to incorporate risk targeted ground motion consistent with ASCE 7-10.

Section 3413A.1.3 – The referenced ASTM requires tension testing of the full size of the rebar. Testing of a reduced section of the rebar with an associated 2” gage length does not account for the effect of the rebar deformations in the rebar strength nor does the 2” gage length of the reduced section meet the 8” gage length required under ASTM A615 and A706.

Repair of the sampled structural member, slab or wall will restore the structural member, slab or wall to its pre-tested condition and, therefore, will not result in weak point in the member where a failure may initiate.

Section 3413A .1.30 – This section is revised for consistency with the California Administrative Code, Chapter 6.

Sections 3416A & 3417A – These sections are revised extensively, so that building that receive extension of NPC-3 deadlines under SB 499 will not have to upgrade some of the existing services/systems, utilities, and egress. This section is supposed to maintain or increase the current degree of public safety, health, and general welfare in existing buildings while permitting repair, alteration, addition, and change of occupancy without requiring full compliance with new building provisions of the code.

Sections 3418A – This section codify the OSHPD Code Application Notice (CAN) 2-3406A for removal of acute care services buildings from OSHPD jurisdiction.

Title 24, Part 2, Volume 2

Chapter 35 - Referenced Standards

References in this chapter are revised for consistency with amendments in all other Chapters.

Title 24, Part 2, Volume 2

Appendix J – Grading

Section J112 – This section is added to permit vibro stone column for ground improvement.

TECHNICAL, THEORETICAL, AND EMPIRICAL STUDY, REPORT, OR SIMILAR DOCUMENTS:

2012 IBC: International Building Code.

ASCE 7-10: Minimum Design Loads for Buildings and Other structures.

ACI 318-11: Building Code Requirements for Structural Concrete and Commentary.

AISC 360-10: Specification for Structural Steel Buildings

AISC 341-10: Seismic Provisions for Structural Steel Buildings

AISC 358-10/358S1-11: Prequalified Connections for Special and Intermediate Steel Moment Frames for Seismic Applications including Supplement No. 1

TMS 402-11: Building Code Requirements for Masonry Structures.

TMS 602-11: Specification for Masonry Structures.

AWC NDS-11: National Design Specification (NDS) for Wood Construction.

AWC SDPWS-2008: Special Design Provisions for Wind and Seismic.

STATEMENT OF JUSTIFICATION FOR PRESCRIPTIVE STANDARDS:

Health and Safety Code (H&SC) Section 18941 requires consistency with state and nationally recognized standards for building construction in view of the use and occupancy of each structure to preserve and protect the public health and safety

CONSIDERATION OF REASONABLE ALTERNATIVES

Government Code Section 11346.2(b)(5)(A) requires a description of reasonable alternatives to the regulation and the agency's reasons for rejecting those alternatives. In the case of a regulation that would mandate the use of specific technologies or equipment or prescribe specific action or procedures, the imposition of performance standards shall be considered as an alternate. It is not the intent of this

paragraph to require the agency to artificially construct alternatives or describe unreasonable alternatives.)

The alternative to these proposed regulations would be to leave regulations as they are which will be inconsistent with H&SC 18941 requirements.

REASONABLE ALTERNATIVES THE AGENCY HAS IDENTIFIED THAT WOULD LESSEN ANY ADVERSE IMPACT ON SMALL BUSINESS.

There will be no adverse impact on small business.

FACTS, EVIDENCE, DOCUMENTS, TESTIMONY, OR OTHER EVIDENCE OF NO SIGNIFICANT ADVERSE IMPACT ON BUSINESS.

The regulations proposed will have no overall cost impact on business, since they are equivalent to current requirements in the Code. Technical update to the national standards for structural design are incorporated, mostly by reference.

DUPLICATION OR CONFLICTS WITH FEDERAL REGULATIONS

These regulations do not duplicate or conflict with federal regulations.